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COORD.

CONST

STRUCTURAL SYMBOLS

CONCRETE (CAST-IN-

CONCRETE BLOCK (CMU)

WELD ALL AROUND

BEAM CONNECTION

TOP OF XXXX

INDICATES

CONNECTION

AT COLUMN AT BEAM

MOMENT

EL. X'-X"

- INDICATES SHEAR SPLICE

-SECTION NUMBER

STRUCTURAL ABBREVIATIONS

-ANCHOR BOLT

-ADDITIONAL

-BEAM

-CLEAR

-COLUMN

-DOUBLE

-DIAGONAL

-DIAMETER

-DETAIL

-CONCRETE

-BEARING

-BASEMENT

-BASEMENT

-CAST IN PLACE

-CONTROL JOINT

-CENTER LINE

-CONNECTION

-CONSTRUCTION

-CONTINUOUS

-COORDINATE

— SHEET CONDITION IS DETAILED

-ABOVE FINISHED FLOOR

-BELOW FINISHED FLOOR

-ARCHITECT/ARCHITECTURAL

-CONCRETE MASONRY UNIT

-DEFORMED BAR ANCHOR

-CONSTRUCTION JOINT

INDICATES SPLICE W/ FULL PENETRATION

SAND/MORTAR/PLASTER

STEEL CROSS SECTION

COMPACTED SOIL (FINISH GRADE)

<u>STRUCTURAL NOTES</u>

STRUCTURAL NOTES — GENERAL

- 1. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE DRAWINGS OF ALL OTHER DISCIPLINES AND THE SPECIFICATIONS. THE CONTRACTOR SHALL VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO INSERTS, ANCHORS, SLEEVES, AND OTHER ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK.
- 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH ALL SAFETY PRECAUTIONS AND REGULATIONS DURING THE WORK INCLUDING TEMPORARY LATERAL BRACING.
- 3. ALL PROPRIETARY COMPONENTS SHALL BE INSTALLED PER THE MANUFACTURERS REQUIREMENTS.

STRUCTURAL DESIGN CRITERIA

BUILDING CODES: UFC 3 301 01 "STRUCTURAL DESIGN" AND THE INTERNATIONAL BUILDING CODE 2012 AS REFERENCED

FLOOR LIVE LOADS:

FLOOR LOADS AND FUTURE FLOORS: 100 PSF

MECHANICAL / ELECTRICAL ROOMS: 150 PSF

FLOOR CONCENTRADED LOADS: 2000 LBS EVERY 2.5 SF

ROOF LIVE LOAD: 20 PSF

FLOOR DEAD LOADS: WEIGHT OF MATERIALS

ROOF DEAD LOAD: WEIGHT OF MATERIALS

Ss = 0.40, S1 = 0.16

Sds = 0.32, Sd1 = 0.18

WIND LOADS: BASIC WIND SPEED (STRENGTH LEVEL)

RISK CATEGORY

WIND EXPOSURE

INTERNAL PRESSURE COEFFICIENT,

SEISMIC LOADS: SITE CLASS

Vult = 120 MPH

= IV

= B

GCPi = +-0.18

SEISMIC FORCE RESISTING SYSTEM:

ORDINARY STEEL MOMENT FRAMES

R = 3.5 $\Omega o = 3.0$ Cd = 3.0

SEISMIC DESIGN CATEGORY

DESIGN PRESSURE FOR OPENINGS:

OPENINGS MORE THAN 4'-0" FROM CORNERS : 30 PSF OPENINGS LESS THAN 4'-0" FROM CORNERS : 35 PSF ROOF DESIGN PRESSURES (AT ASD LEVEL):

 ZONE 1 (> 3 FT. FROM EDGES)
 : 23 PSF

 ZONE 2 (< 3 FT. FROM EDGES)</td>
 : 39 PSF

 ZONE 3 (< 3 FT. FROM CORNERS)</td>
 : 58 PSF

 OVERHANGS
 : 51 PSF

SUBMITTALS

-DIMENSION

-EACH FACE

-ELEVATION

-ENGINEER

-EXPANSION JOINT

-EDGE OF DECK

-EDGE OF SLAB

-FINISHED FLOOR

-FINISHED FLOOR

-DRAWING

 $-\mathsf{EACH}$

-EQUAL

-EQUIPMENT

-EACH WAY

-EXPANSION

ELEVATION

-FOUNDATION

-GAUGE/GAGE

-HORIZONTAL

-ON CENTER

-REFERENCE

SQ. FT. -SQUARE FOOT/FEET

OTHERWISE

-UNLESS NOTED

-SIMILAR

-TOP OF

-GALVAÑIZE (D) (ING)

-FINSH(ED)

-FLOOR

-FOOTING

-HIGH

 $-\mathsf{METAL}$

-EXISTING

-EXTERIOR

EQ

EXT

F.F.

FND.

FTG.

GΑ

U.N.O.

- 1. AS A MINIMUM THE FOLLOWING SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW:
- * CONCRETE MIX DESIGN.
- * LIGHT GAUGE STEEL SHOP DRAWINGS* STRUCTURAL STEEL SHOP DRAWINS
- * BRICK MASONRY MORTAR MIS DESIGN (S)
 * EPOXY SYSTEM SUBMITTALS
- * REBAR SHOP DRAWINGS
- 2. SPECIAL INSPECTIONS AND STRUCTURAL OBSERVATION PROCEDURES SHALL BE AS DEFINED IN CHAPTER 17 OF THE 2012 INTERNATIONAL BUILDING CODE.
- 3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROCURING AN INDEPENDENT THIRD PARTY INSPECTOR TO PERFORM THE SPECIAL INSPECTIONS FOR THIS WORK. THE CONTRACTOR SHALL COORDINATE AND SCHEDULE THE WORK TO ALLOW FOR REQUIRED SPECIAL INSPECTIONS WITHOUT DELAYING THE PERFORMANCE OF THE WORK. A MINIMUM OF 48 HOURS ADVANCE NOTICE SHALL BE PROVIDED TO SPECIAL INSPECTORS BY THE CONTRACTOR.

SUBSTITUTIONS

- 1. MANUFACTURER'S LISTED (I.E. SIMPSON, HILTI, ETC) ARE USED AS THE BASIS FOR DESIGN AND MAY BE SUBSTITUTED WITH AN APPROVED EQUAL PRODUCT PROVIDED THE SUBSTITUTION HAS EQUAL OR BETTER DESIGN PROPERTIES.
- 2. SUPPLIER CONTACT INFORMATION:
 THE FOLLOWING SUPPLIERS, WHERE SPECIFIED HEREIN, MAY BE CONTACTED AS INDICATED BELOW.
- * SIMPSON STRONG TIE: (800) 999-5099 www.strongtie.com
- * HILTI: (800) 879-8000 www.us.hilti.com

FOUNDATION NOTES

- 1. ALL FOOTINGS SHALL BEAR ON UNDISTURBED, FIRM NATURAL SOIL, OR COMPACTED FILL CAPABLE OF SUPPORTING A DESIGN BEARING PRESSURE OF 1,500 PSF. ALL FOUNDATION EXCAVATIONS SHALL BE EVALUATED BY THE TESTING AGENCY PRIOR TO POURING CONCRETE.
- 2. UNLESS OTHERWISE NOTED, PROVIDE THE FOLLOWING COVER FOR FOUNDATION REINFORCEMENT:
- 3. BOTTOM BARS & BARS IN CONCRETE CAST AGAINST EARTH: 3"
- 4. BARS THAT ARE EXPOSED TO WEATHER:
 #5 OR SMALLER 1 1/2"
 #6 OR BIGGER 2"
- 5. ALL BARS SHALL BE LAPPED 40 X THE BAR DIAMETER AT SPLICES
- 6. PRIOR TO COMMENCING FOUNDATION WORK, COORDINATE WORK WITH UTILITIES.

CAST-IN-PLACE CONCRETE NOTES

- 1. CONCRETE MIXES SHALL BE DESIGNED PER ACI 301, USING PORTLAND CEMENT CONFORMING TO ASTM C-150 OR C-595, AGGREGATE CONFORMING TO ASTM C-33, AND ADMIXTURES CONFORMING TO ASTM C-494, C-1017, C818, AND C-260. CONCRETE SHALL BE READY MIXED IN ACCORDANCE WITH ASTM C-94.
- 2. CONCRETE SHALL CONFORM TO THE FOLLOWING:

LOCATIONMIN f'cSLABS: 4000 PSIFOOTINGS: 4000 PSI

- 3. REINFORCING STEEL, INCLUDING HOOKS AND BENDS, SHALL BE DETAILED IN ACCORDANCE WITH ACI 315. ALL REINFORCING STEEL INDICATED AS BEING CONTINUOUS SHALL BE LAPPED WITH A TYPE 2 SPLICE UNLESS OTHERWISE NOTED.
- 4. BAR SUPPORTS SHALL BE PROVIDED FOR ALL REINFORCING STEEL TO ENSURE MINIMUM CONCRETE COVER. BAR SUPPORTS SHALL BE PLASTIC TIPPED OR STAINLESS STEEL.
- 5. CONCRETE EXPOSED TO WEATHER SHALL BE AIR ENTRAINED TO 5% $(\pm 1\%)$ WITH AN ADMIXTURE THAT CONFORMS TO ASTM C-260.

SLAB ON GRADE NOTES

- 6. PROVIDE CONCRETE SLABS OVER 10 MIL POLYETHYLENE VAPOR BARRIER AND 4" OF POROUS FILL AS FOLLOWS: 4"SLAB REINFORCED WITH 6x6-W2.1xW2.1 WELDED WIRE FABRIC AND WITH 4,000 PSI MIX CONCRETE
- 7. MAXIMUM SLUMP FOR CONCRETE SLABS WILL BE 5" WITH TYPE II CEMENT.
- 8. ALL WELDED WIRE FABRIC SHALL BE IN ACCORDANCE WITH ASTM A-185. LAP ADJOINING PIECES AT LEAST ONE FULL MESH. WELDED WIRE FABRIC SHALL BE ORDERED IN SHEETS, NOT ROLLS. WELDED WIRE FABRIC SHALL BE BLOCKED INTO POSITION WITH PRECAST CONCRETE BLOCKS HAVING THE SAME COMPRESSIVE STRENGTH OF THE SLAB.
- 9. THE ALTERNATE WIRES OF THE WELDED WIRE FABRIC MUST BE PRECUT AT THE SLAB CONTRACTION JOINT LOCATIONS TO CREATE A "WEAKENED PLANE".
- 10. THE USE OF POLYPROPYLENE FIBERS (IN LIEU OF WELDED WIRE FABRIC) IS PROHIBITED.
- 11. ALL POROUS FILL MATERIAL SHALL BE A CLEAN GRANULAR FILL MATERIAL WITH 100% PASSING THE 1½" SIEVE AND NO MORE THAN 5% PASSING THE NO. 4 SIEVE. POROUS FILL SHALL BE COMPACTED TO 98% MAX DRY DENSITY PER ASTM D-1557 MODIFIED PROCTOR METHOD.
- 12. SLAB JOINTS SHALL BE FILLED WITH A SEALANT PER THE MANUFACTURER RECOMMENDATIONS.
- 13. SLABS EXPOSED TO WEATHER SHALL BE AIR ENTRAINED TO 5% $(\pm 1\%)$ WITH AN ADMIXTURE THAT CONFORMS TO ASTM C-260.
- 14. THE SLAB SHALL BE WET CURED BY KEEPING THE SLAB MOIST FOR A PERIOD OF SEVEN DAYS. ALTERNATIVELY, PROVIDE A WET-CURING SEALANT PER THE MANUFACTURERS RECOMMENDATIONS.

CONSTRUCTION JOINTS

- 1. ALL CONSTRUCTION JOINTS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE APPLICABLE CODE SECTION AND THE TYPICAL CONSTRUCTION JOINT DETAILS SHOWN ON THE STRUCTURAL DRAWINGS.
- 2. ALL SURFACES OF CONSTRUCTION JOINTS SHALL BE CLEANED TO REMOVE DUST, CHIPS, OR OTHER FOREIGN MATTER PRIOR TO PLACING THE ADJACENT CONCRETE.
- 3. THE CONTRACTOR SHALL SUBMIT THE PROPOSED LOCATIONS OF CONSTRUCTION JOINTS FOR APPROVAL BY THE STRUCTURAL ENGINEER BEFORE STARTING CONSTRUCTION.

METAL DECK

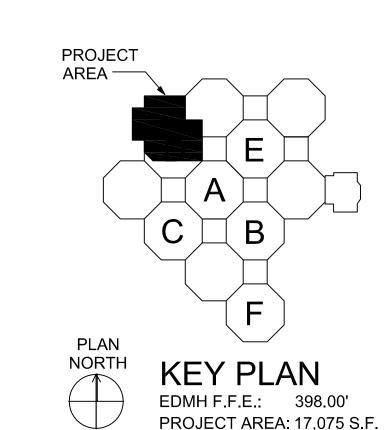
- 1. INSTALL DECKING PER AWS S1.3, AND FOLLOW ANSI/SDI QA/QC-2001 STANDARD FOR QUALITY CONTROL AND QUALITY ASSURANCE FOR INSTALLATION OF STEEL DECK TO PROMOTE QUALITY IN DECK INSTALLATION
- 2. MECHANICAL FASTENERS OF EQUAL CAPACITY ARE ACCEPTABLE TO WELDED CONNECTIONS SHOWN

EMBEDDED ITEMS AND ANCHORS FOR CONCRETE AND MASONRY

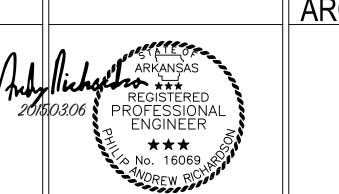
- 1. ALL BOLTS OR REINFORCING BAR DOWELS ATTACHED TO EXISTING CONSTRUCTION SHALL BE EPOXY GROUTED INTO EXISTING CONCRETE U.O.N. EPOXY GROUTED ANCHORS SHALL HAVE EQUIVALENT DESIGN VALUES TO THE PUBLISHED DESIGN VALUES BY HILTI HIT RE-500 (ICBO REPORT No. ESR-2322). SUBMIT TECHNICAL DATA TO STRUCTURAL ENGINEER FOR APPROVAL.
- 2. WHEN INSTALLING ANCHOR BOLTS, INSERTS, OR DOWELS INTO EXISTING CONCRETE, USE A MECHANICAL REBAR LOCATING DEVICE TO LOCATE EXISTING REINFORCING AND DRILL HOLE TO MISS REINFORCEMENT.
- 3. ALL ANCHORS NOTED AS EXPANSION (EXP) ANCHORS SHALL HAVE EQUIVALENT DESIGN VALUES TO THE PUBLISHED DESIGN VALUES TO HILTI KWIK BOLT—TZ (ICBO REPORT No. ESR—1917). SUBMIT TECHNICAL DATA TO THE STRUCTURAL ENGINEER FOR APPROVAL.

STRUCTURAL STEEL

- 1. ALL DESIGN, FABRICATION AND ERECTION SHALL CONFORM TO THE AISC STEEL CONSTRUCTION MANUAL. WELDING SHALL CONFORM TO THE LATEST AWS AND AISC SPECIFICATIONS.
- 2. WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE BEST PRACTICE AND WITHIN THE TOLERANCES SPECIFIED IN THE AISC SPECIFICATIONS FOR STRUCTURAL STEEL.
- 3. IT IS SPECIFICALLY NOTED THAT BURNED HOLES ARE NOT ACCEPTABLE UNLESS SPECIAL PERMISSION IS GIVEN BY ENGINEER.
- 4. ALL SHOP FABRICATED WORK SHALL BE DONE IN A SHOP APPROVED BY THE GOVERNING AGENCY. FABRICATOR SHALL SUBMIT PROGRAM OF WELDING INSPECTION TO ENGINEER FOR APPROVAL.
- 5. ALL STRUCTURAL STEEL SHALL BE AS FOLLOWS UNO: ALL WF. WT SHAPES A992 GRADE 50 ASTM A36 CONNECTION PL AND MISC. STEEL GUSSET AND COLLECTOR PLATES ASTM A572 GRADE 50 PIPE COLUMNS (TYPE S, SEAMLESS) ASTM A53 GRADE B ASTM A500 GRADE B STRUCTURAL TUBING ANGLE, CHANNELS ASTM A36 THREADED ROD ASTM A36 STAINLESS STEEL TYPE 316, 50 KSI
- 6. ALL HIGH STRENGTH BOLTS SHALL BE ASTM A325—N TYPE UNLESS OTHERWISE
- 7. ALL BOLTS USED FOR ERECTION SHALL BE ASTM A325 TYPE WITH THREADS EXCLUDED FROM SHEAR PLANES.
- 8. ALL PLAIN ANCHORS SHALL BE A36; ALL ANCHOR BOLTS SHALL COMPLY WITH ASTM F1554. 3" MINIMUM CONCRETE COVER WILL BE PLACED AROUND ALL ANCHOR BOLTS EXPOSED TO THE WEATHER, U.N.O.
- 9. WELDING MATERIALS: PROVIDE TYPE REQUIRED FOR MATERIALS BEING WELDED, PER AWS D1.1.
- 10. PROVIDE CONTINUOUS INSPECTION FOR ALL FABRICATION AND WELDING OF STRUCTURAL STEEL IN ACCORDANCE WITH CODE REQUIREMENTS. ALL COMPLETE PENETRATION GROOVE WELDS IN JOINTS AND SPLICES SHALL BE TESTED 100 PERCENT IN ACCORDANCE WITH IBC. USE ONE OF THE APPROVED METHODS OF TIGHTENING HIGH STRENGTH BOLTS.
- 11. A WELDING SEQUENCE SHALL BE PLANNED TO MINIMIZE RESIDUAL STRESSES AND DISTORTIONS OF INDIVIDUAL MEMBERS AND THE BUILDING FRAME. ALL DETAILING, FABRICATION, AND ERECTION SHALL COMPLY WITH AISC, LATEST
- 12. UNLESS OTHERWISE NOTED, ALL STIFFENER PLATES ARE 3/8" THICK MINIMUM AND ALL BUTT WELDS ARE FULL PENETRATION WELDS. ERECTION CLIPS, TEMPORARY BRACING, ETC., REQUIRED BY THE CONTRACTOR ARE NOT SHOWN.
- 13. SUBMIT SHOP DRAWINGS FOR THE FABRICATION AND ERECTION OF ALL ASSEMBLIES OF STRUCTURAL STEEL WORK. INCLUDE PLANS AND ELEVATIONS AT NOT LESS THAN 1/8" TO 1'-0" SCALE, AND INCLUDE DETAILS OF SECTIONS AT NOT LESS THAN 1" TO 1'-0" SCALE.
- 14. NO FINISH FABRICATION SHALL BE COMMENCED OR MATERIAL DELIVERED TO THE JOB UNTIL THE ENGINEER HAS REVIEWED AND APPROVED THE SHOP DRAWINGS.
- 15. ALL STRUCTURAL STEEL SHALL BE PAINTED WITH ONE SHOP COAT OF ZINC CHROMATE PRIMER OR EQUAL. AFTER ERECTION, FIELD CONNECTIONS SHALL BE TOUCHED UP. DO NOT PAINT PORTION OF STEEL TO BE EMBEDDED IN CONCRETE, HEADED ANCHOR STUDS, FAYING SURFACES OR AREAS TO RECEIVE FIRE PROOFING. EXTERIOR, EXPOSED STEEL MEMBERS ARE SPECIFIED TO BE HOT—DIPPED GALVANIZED OR STAINLESS STEEL.
- 16. WELD LENGTHS CALLED FOR ON PLANS ARE THE NET EFFECTIVE LENGTH REQUIRED. WHERE FILLET WELD SYMBOL IS GIVEN WITHOUT INDICATION OF SIZE, USE MINIMUM SIZE WELDS AS SPECIFIED IN AISC MANUAL OF STEEL CONSTRUCTION LATEST EDITION. THIS INCLUDES OPEN WEB JOIST CONNECTIONS.
- 17. THE USE OF E70T-4 WELDING WIRE IS NOT ALLOWED FOR ANY APPLICATION.



CONSTRUCTION DOCUMENT SUBMISSION FULLY SPRINKLERED





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Approved: Project Director

EMERGENCY DEPARTMENT
MENTAL HEALTH
IMPROVEMENTS

Location
LITTLE ROCK, ARKANSAS

2015.03.06

Project Number
CSI-112
Building Number
1
Drawing Number
SS101

Department of Veterans Affairs

CENTRAL ARKANSAS

VETERANS AFFAIRS

HEALTHCARE

SYSTEM

Revisions: Date

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DESIGN/BUILD Fax: 251.990.3716

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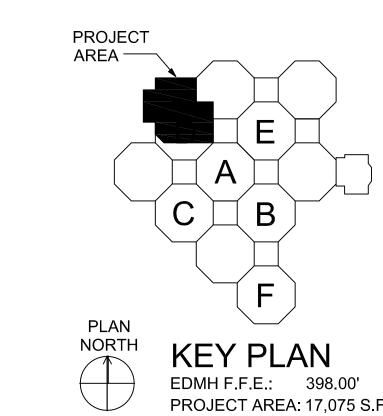
- 16. WRITTEN WELDING PROCEDURE SPECIFICATIONS (WPS) PER THE RECOMMENDATIONS OF THE AMERICAN WELDING SOCIETY (AWS) SHALL BE DEVELOPED BY THE FABRICATOR/ERECTOR AND SUBMITTED FOR REVIEW BY THE ENGINEER PRIOR TO ANY WELDING OF THE STRUCTURAL STEEL. THE WELDING PROCEDURES SHALL INCLUDE ALL THE WELDED JOINTS AND CONFIGURATIONS TO BE USED ON THIS PROJECT—ONLY WPS WHICH ARE RELEVANT TO THIS PROJECT SHALL BE SUBMITTED. ALL WELDED JOINTS SHALL BE PRE—QUALIFIED PER AWS OR BE QUALIFIED BY TEST PER AWS. A PROCEDURE QUALIFICATION RECORD (PQR) SHALL BE INCLUDED WITH THE WPS IF THE WELDING PROCEDURE OR JOINT IS QUALIFIED BY TESTING. THE ELECTRODE MANUFACTURER AND PRODUCT/TRADE NAME SHALL BE IDENTIFIED IN THE WPS IN ADDITION TO THE AWS ELECTRODE CLASSIFICATION NAME. A COPY OF THE ELECTRODE MANUFACTURER'S TECHNICAL DATA SHEETS WITH THE RECOMMENDED WELDING PARAMETERS SHALL BE SUBMITTED WITH THE WPS.
- 17. DO NOT WELD ANY STRUCTURAL STEEL MEMBER OR CONNECTION UNLESS EXPLICITLY CALLED OUT IN THE CONTRACT DOCUMENTS.
- 18. WELD SYMBOLS SHOW FINAL WELD REQUIRED. THE CHOICE TO WELD IN THE FIELD OR IN THE SHOP SHALL BE UP TO THE CONTRACTOR AND SHALL BE INDICATED IN THE FABRICATOR'S SHOP DRAWINGS.
- 19. ALL STRUCTURAL STEEL SHALL BE PROPERLY GUYED AND BRACED UNTIL FLOOR AND ROOF FRAMING SYSTEM AND LATERAL LOAD RESISTING SYSTEM IS IN PLACE.
- 20. THIS STRUCTURE IS CONSIDERED A NON-SELF-SUPPORTING STEEL FRAME. THE CONTRACTOR SHALL PROVIDE ADEQUATE TEMPORARY SUPPORTS UNTIL ALL PERMANENT SHEAR WALLS, MOMENT FRAMES, BRACED FRAMES, AND FLOOR SLABS ARE IN PLACE.
- 21. DETAIL STEEL BEAM CONNECTIONS AS SIMPLE SPAN BEAMS, UNLESS OTHERWISE NOTED.
- 22. ALL BEAM CONNECTIONS SHALL BE DETAILED TO PROVIDE A SHEAR CONNECTION WITH A MINIMUM DESIGN CAPACITY AS THAT SHOWN IN THE DRAWINGS BEAM SHEAR TAB CONNECTION TABLE FOR THE CORRESPONDING BEAM SIZE.
- 23. ALL CONNECTIONS NOT DETAILED OR OTHERWISE NOTED SHALL BE DESIGNED AS AISC TYPE 2 BOLTED CONNECTIONS DESIGNED FOR FULL LOAD CAPACITY OF THE CONNECTING MEMBERS. MOMENT CONNECTIONS NOT OTHERWISE NOTED ON THE PLANS SHALL BE DESIGNED AS A TYPE 1 (FULLY RESTRAINED) MOMENT CONNECTION WITH FULL MOMENT CAPACITY OF THE MEMBER.
- 24. ALL BOLTS IN MOMENT CONNECTIONS SHALL BE BEARING TYPE CONNECTIONS.
- 25. ALL BOLTS IN HANGER CONNECTIONS SHALL BE PRE-TENSIONED.
- 26. ALL WELDS SHALL BE MADE BY WELDERS CERTIFIED ACCORDING TO AWS PROCEDURES.

#### METAL STUDS AND JOISTS

- 1. MATERIAL SHALL CONFORM TO ASTM A 446 GRADE D WITH MIN. YIELD POINT OF 50 KSI
- 2. MATERIAL SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A 525, G60 COATING DESIGNATION.
- 3. MATERIAL SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A 525, G60 COATING DESIGNATION.
- 4. WELDING SHALL CONFORM TO AISI & AWS D1.3 SPECIFICATIONS.
- 5. ALL CONNECTIONS SHALL BE SCREWED OR WELDED PER DRAWINGS.
- 6. STUDS SHALL BE INSTALLED AS INDICATED ON DRAWINGS. PROVIDE MIN. ONE JACK STUDS & TWO FULL HEIGHT STUDS AT EACH END OF THE OPENINGS
- 7. STUDS SHALL HAVE A BRIDGING LINE INSTALLED AT A MAXIMUM DISTANCE OF
- 8. THE NOMENCLATURE USED ON DRAWINGS IS PER SSMA & AISI. THE SECTIONS SUPPLIED BY THE MANUFACTURER SHALL MEET OR EXCEED THE STRENGTH OF SPECIFIED MEMBERS.

#### METAL STUDS AND JOISTS (CONT)

- 9. FOR SCREWS 3/4" MINIMUM CLEARANCE MUST BE MAINTAINED FROM ALL EDGES OF THE STEEL MEMBERS. A 3/4" MINIMUM ON CENTER SPACING MUST BE MAINTAINED BETWEEN ADJACENT SCREWS.
- 10. PROVIDE BOTTOM TRACK 18 GA MINIMUM U.N.O.
- 11. PROVIDE (1)#10-16 SCREW EACH FLANGE AT SIDE EACH STUD TO TOP/BOTTOM TRACK U.N.O.
- 12. ALL FIELD CUTTING OF STUDS MUST BE DONE BY SAWING OR SHEARING.
  TORCH CUTTING OF COLD—FORMED MEMBERS IN NOT PERMITTED.
- 13. NO NOTCHING OR COPING OF STUDS IS ALLOWED UNLESS STATED WITHIN THE DRAWINGS PACKAGE.
- 14. END STUDS MUST BE SEATED IN RUNNER TRACK, WHICH MUST HAVE FULL BEARING ON STRUCTURE.
- 15. FRAMING CONTRACTOR IS TO ENSURE PUNCHOUT ALIGNMENT WHEN ASSEMBLING LATERAL BRACING AND FIELD CUTTING STUDS. LATERAL BRACING MUST BE INSTALLED AT THE TIME THE WALL IS ERECTED.
- 16. USE MINIMUM OF THREE STUDS AT THE CORNERS OF ALL EXTERIOR WALLS U.N.O.
- 17. ALL METAL TO METAL SCREW CONNECTIONS ARE BASED ON SECTION E4 OF THE 2012 AISI SPECIFICATION FOR DESIGN OF COLD—FORMED STEEL MEMBERS WHICH OUTLINES PROVISIONS FOR METAL TO METAL SCREW CONNECTIONS.
- 18. POWER DRIVEN FASTENERS SYSTEMS AND EXPANSION ANCHORS SYSTEMS ARE BASED ON LITERATURE PUBLISHED BY HILTI FASTENING SYSTEMS INC. ALTERNATE MANUFACTURE'S FASTENERS OF COMPARABLE SPECIFICATION AND LOAD CAPACITY ARE ACCEPTABLE. PROVIDE SHOP DRAWINGS AND LITERATURE OF ALTERNATES.
- 19. SHOP DRAWINGS SHALL BE PROVIDED FOR LIGHT GAUGE FRAMING, INCLUDING CONNECTIONS. BRIDGING AND BRACING PROVISIONS



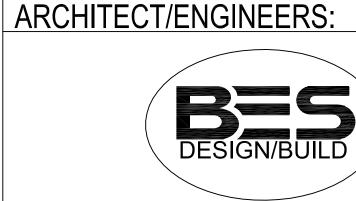
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ARKANSAS

REGISTERED

PROFESSIONAL
ENGINEER

WOREW RICK



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STRUCTURAL NOTES

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CENTRAL ARKANSAS
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HEALTHCARE
SYSTEM

Department Veterans At

VA FORM 08-6231

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2015.03.06

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